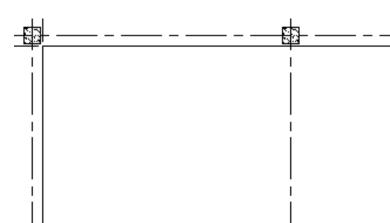
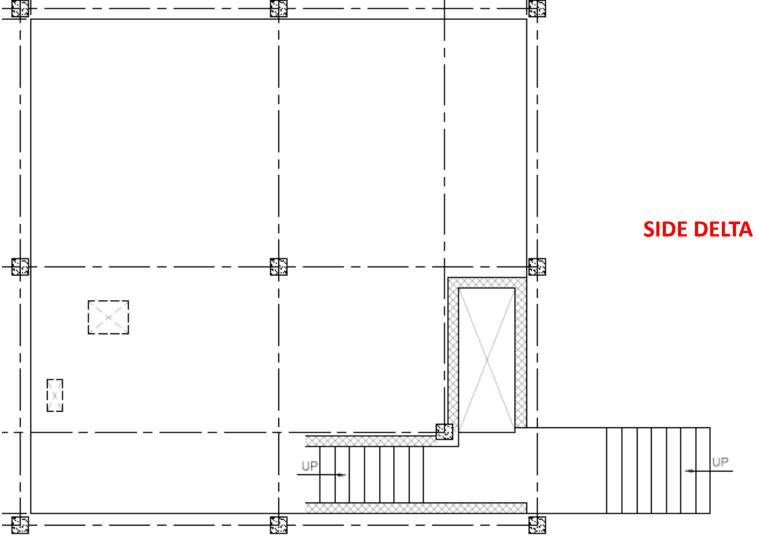
Current Class A CMU Building (not to scale)

SIDE CHARLIE



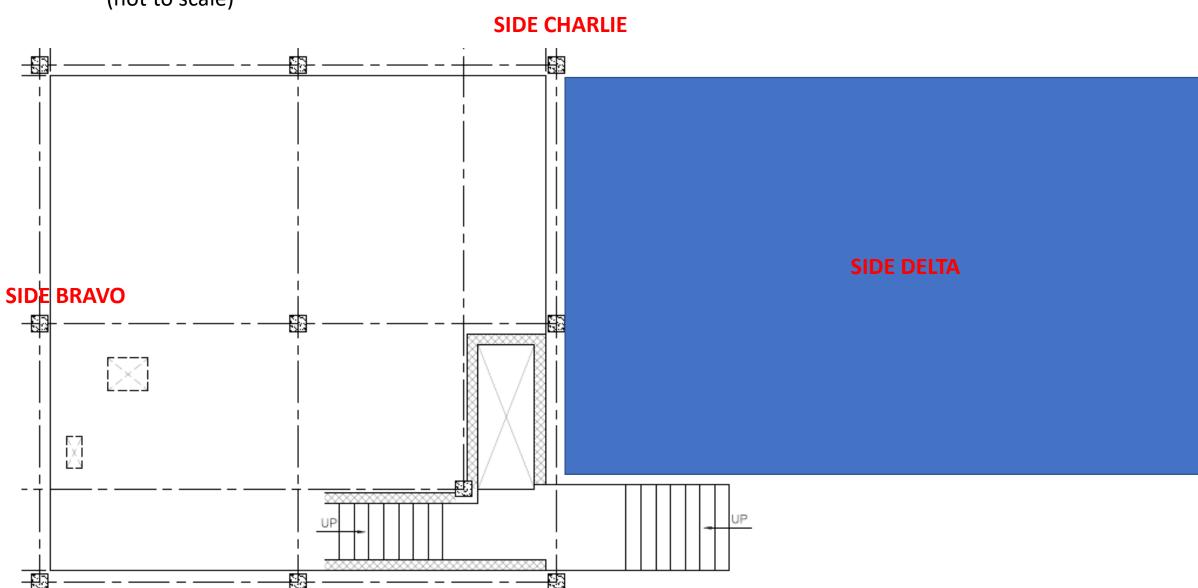
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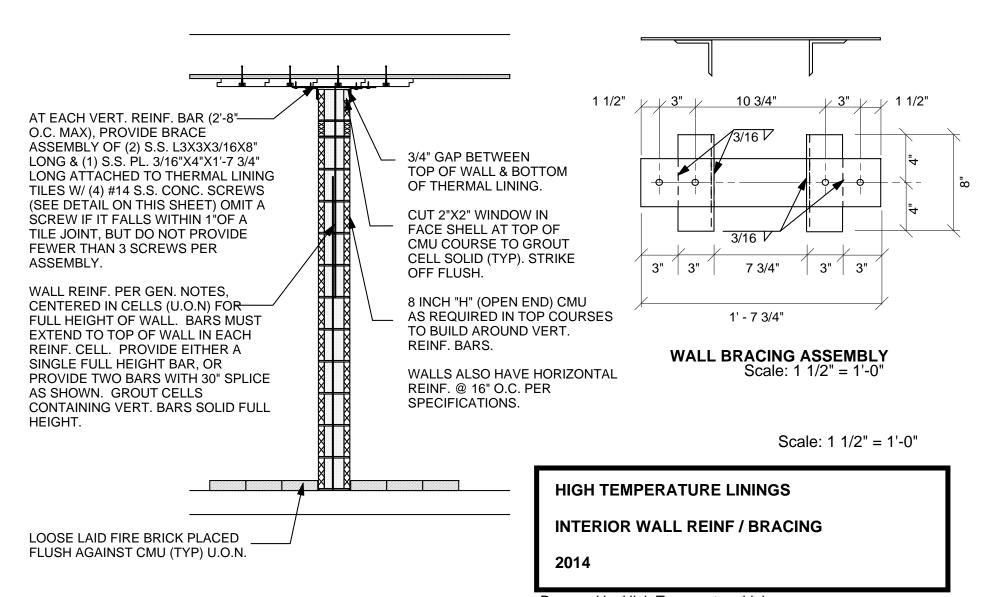
SIDE ALPHA

Shipping Container Add-On Area

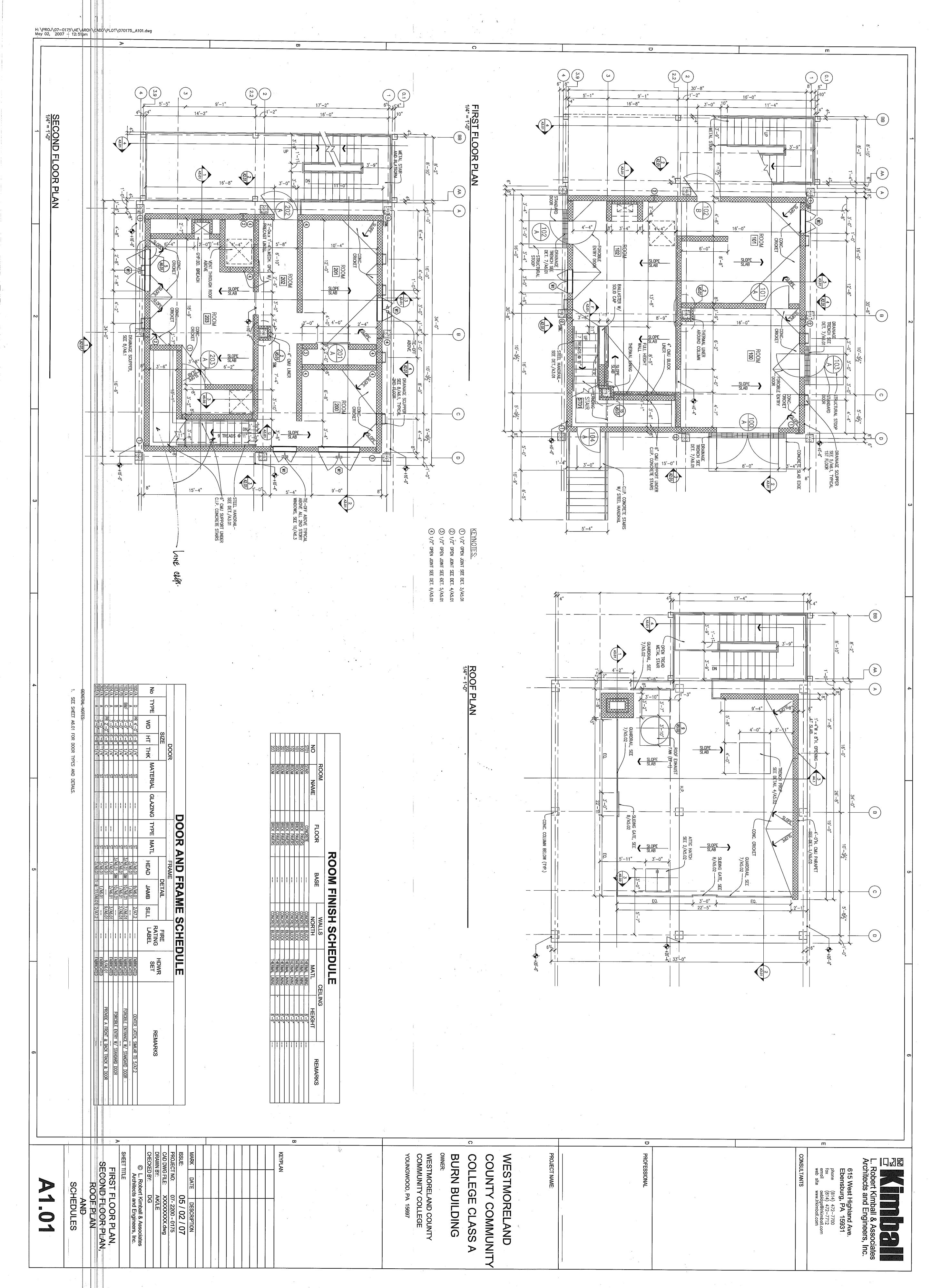
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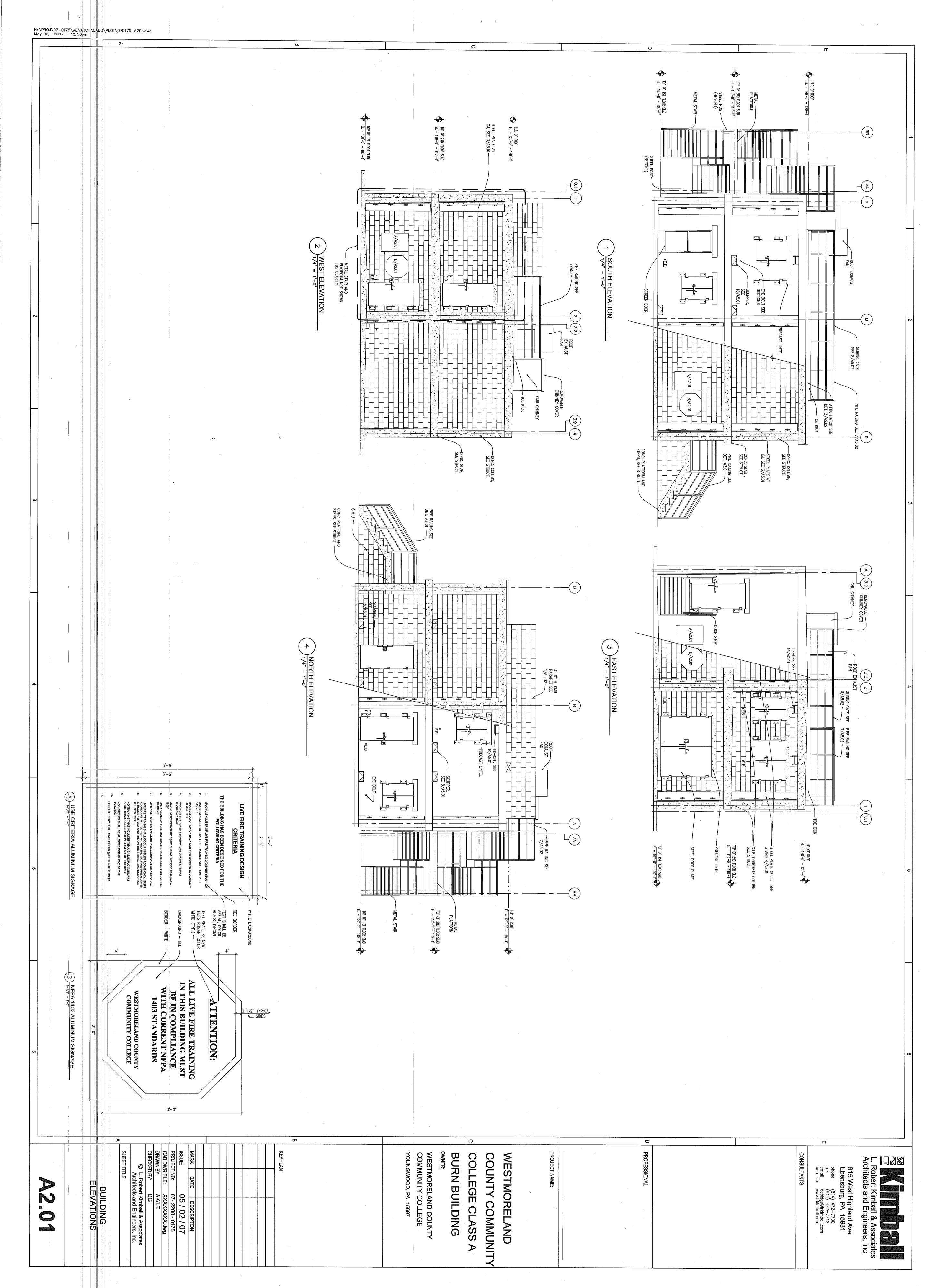


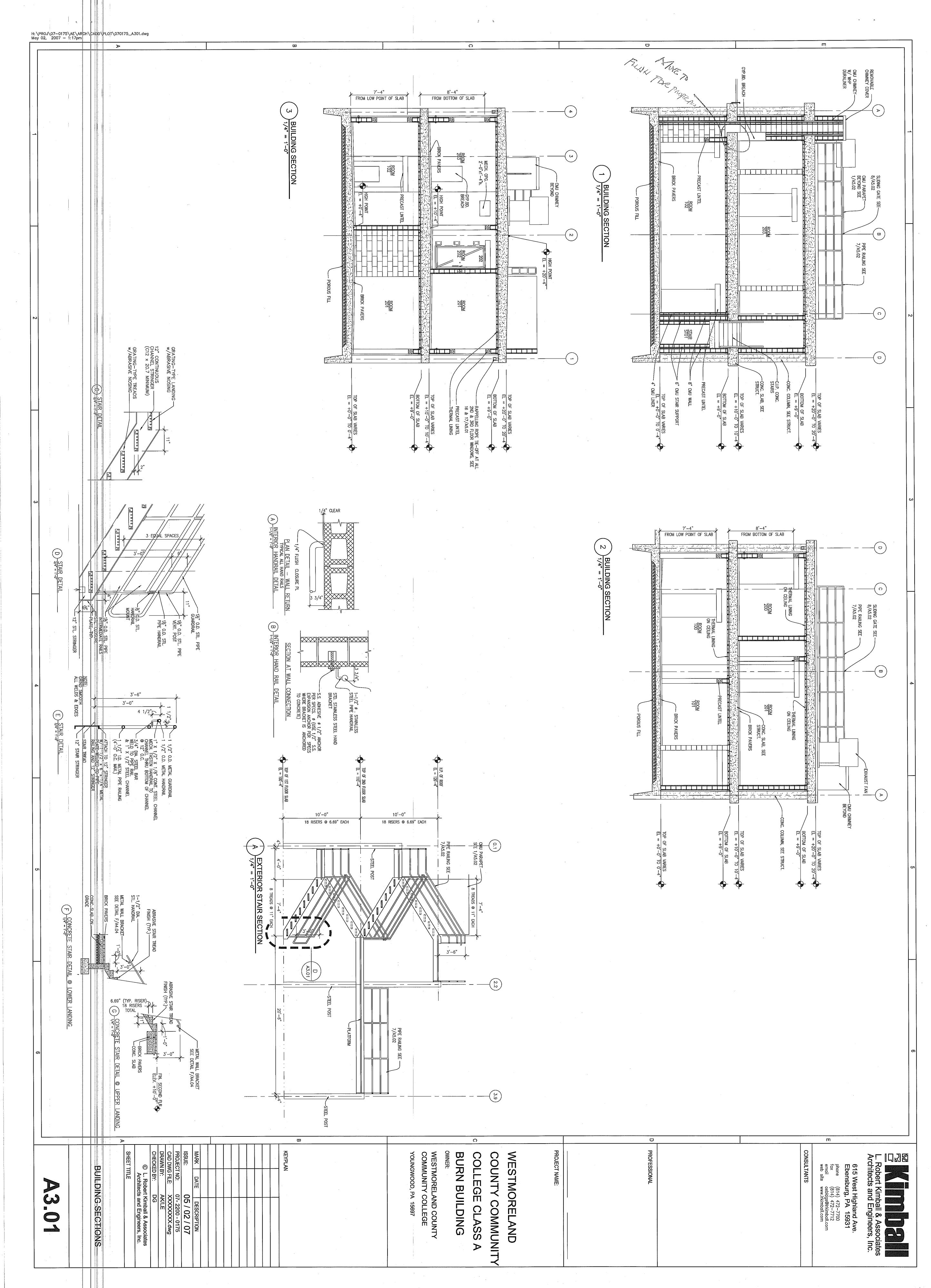
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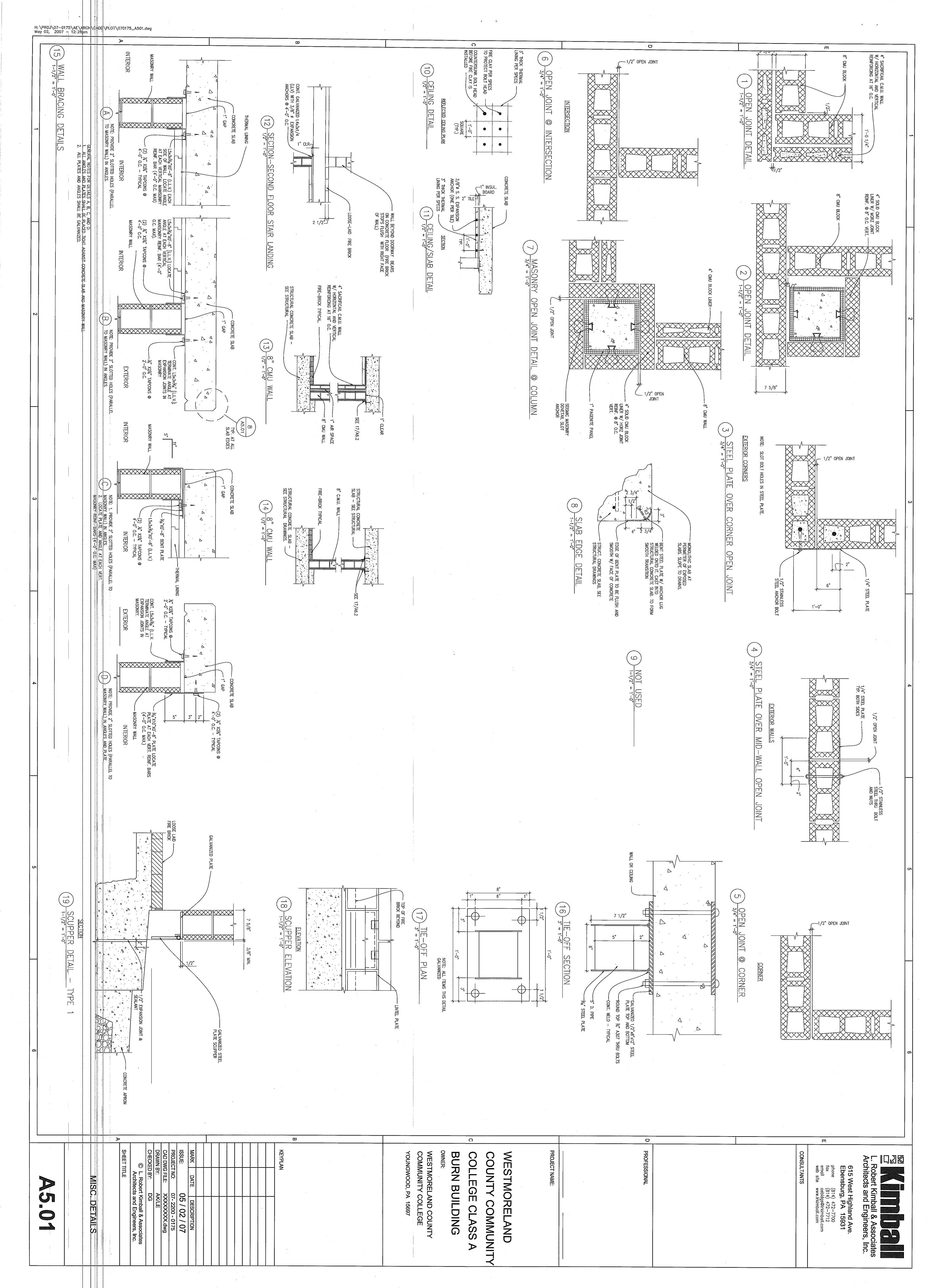


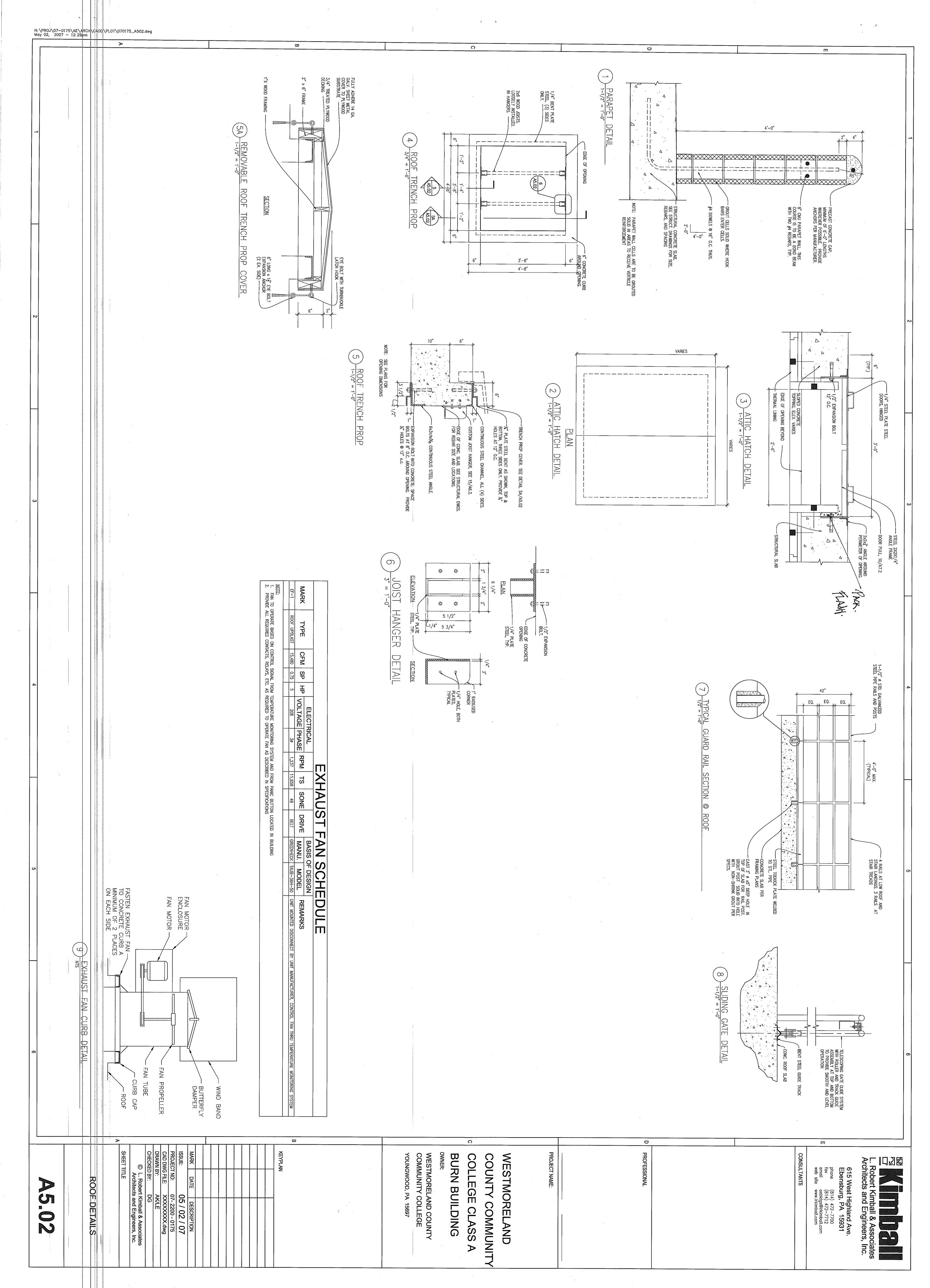
Prepared by High Temperature Linings P. O. Box 1240, White Stone, VA 22578 800-411-6313 / www.firetrain.com

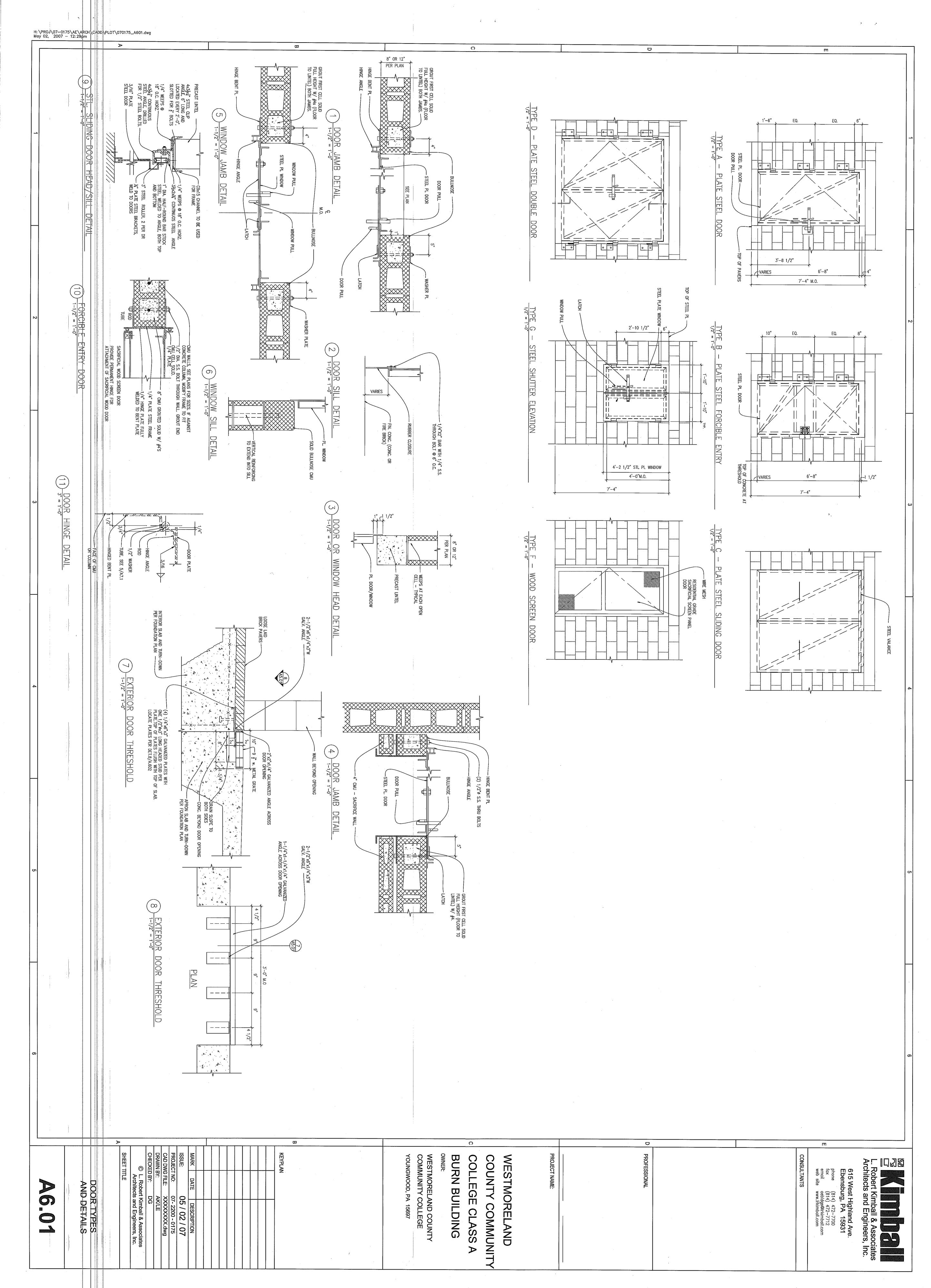


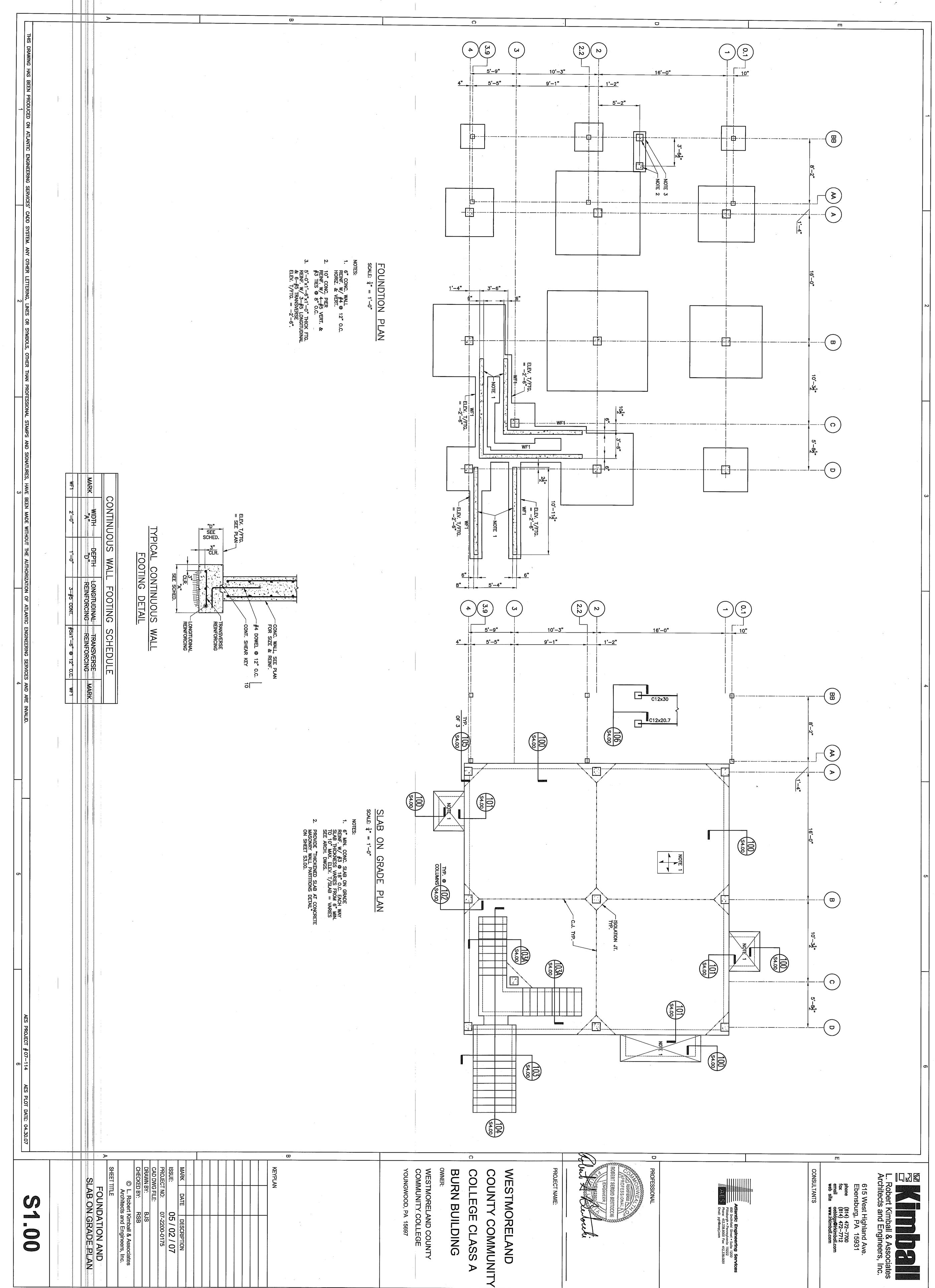




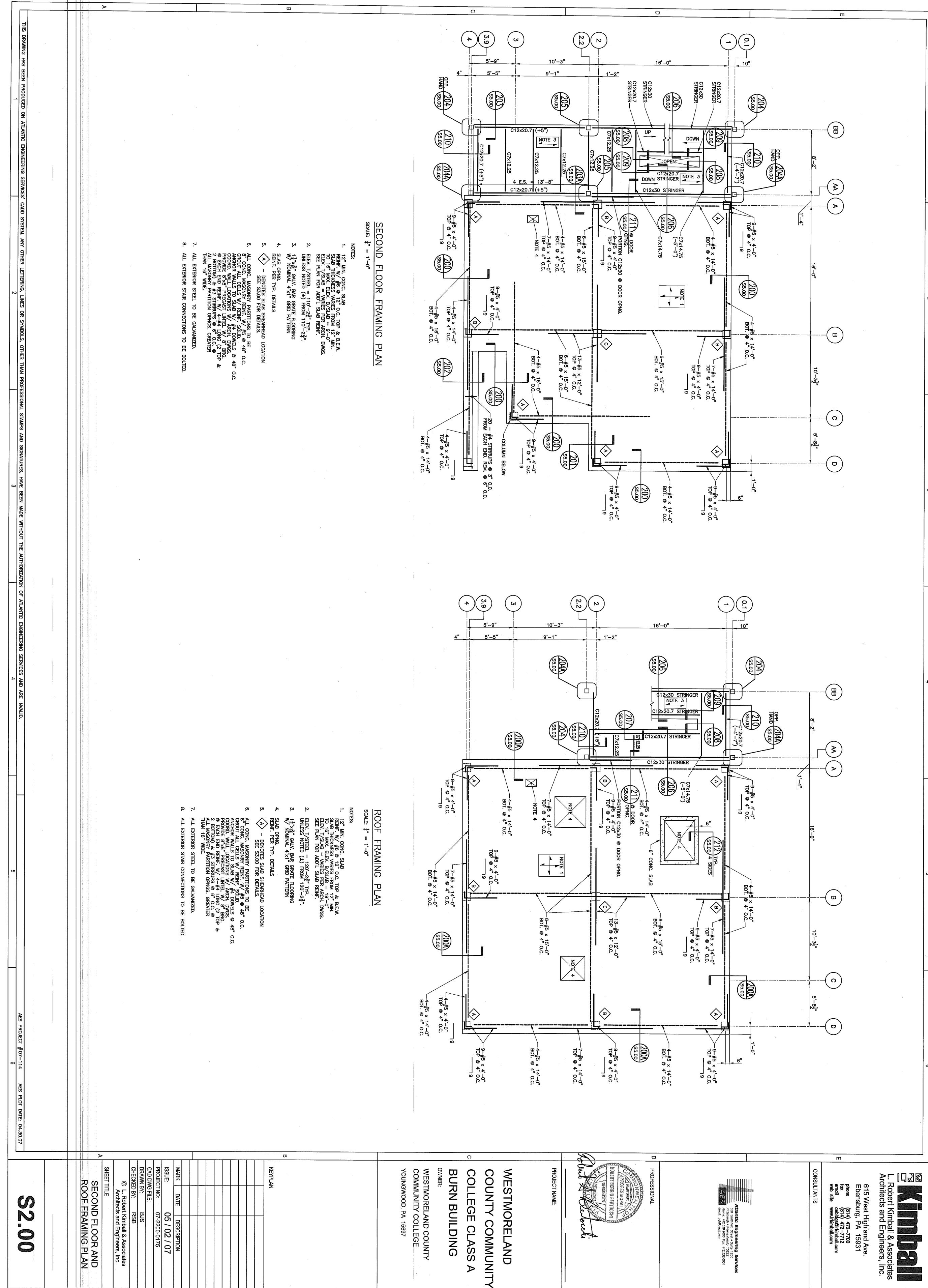








THIS DRAWING HAS BEEN PRODUCED ON ATLANTIC ENGINEERING SERVICES' CADI				PROJECT IS FOR CONFORMANCE WITH THE DESIGN CONCEPT AND FOR GENERAL COMPLIANCE WITH THE DESIGN CONCEPT AND FOR GENERAL COMPLIANCE WITH THE INFORMATION CONTAINED IN THE CONTRACT DOCUMENTS. COMMENTS REGARDING THESE SUBMITTALS DO NOT RELIEVE THE CONTRACTOR FROM COMPLIANCE WITH THE CONTRACT DOCUMENTS. THE CONTRACTOR IS RESPONSIBLE FOR PERFORMING HIS WORK IN A SAFE AND SATISFACTORY MANNER.	PRINTED COPIES OF SHOP DRAWINGS SHALL CONSIST OF TWO (2) NON-REPRODUCIBLE PRINTS AND ONE (1) REPRODUCIBLE TRACING/SEPIA. UPON THE COMPLETION OF THE SHOP DRAWING REVIEW, ONE (1) PRINT AND ONE (1) TRACING/SEPIA SHALL BE RETURNED TO THE PROJECT ARCHITECT FOR DISTRIBUTION TO THE CONTRACTOR.	ATLANTIC ENGINEERING SERVICES, THE CONTRACTOR SHALL BE REQUIRED TO ENTER INTO A WRITTEN AGREEMENT WITH ATLANTIC ENGINEERING SERVICES AND TO PAY A SERVICE FEE TO USE THE STRUCTURAL DRAWINGS IN THE PREPARATION OF SHOP DRAWINGS. 120.4 THE CONTRACTOR SHALL SUBMIT PRINTED COPIES OF SHOP DRAWINGS. DRAWINGS FOR REVIEW BY ATLANTIC ENGINEERING SERVICES.	SUBMITTED FOR THE NOT ATLANTIC NOT ATLANTIC CON OF THE STRUCT PERMISSION OF ATLANTIC REJECTED. IF	TO: MEMBER SIZES AND DIMENSIONS; GRADES TO: MEMBER SIZES AND DIMENSIONS; GRADES ED; MATERIAL PREPARATION REQUIRED; MATERIAL COATINGS TO BE FURNISHED; INFORMATION ROPES AND HOLES REQUIRED FOR OTHER TRADITIONS; CAMBER AND OTHER DEVIATION FROM LIVENTS FOR TEMPORARY STABILIZATION. TRACTOR SHALL NOT DIRECTLY INCORPORATE TRADITIONS THEREOF INTO SERVICE OF THE PORTIONS T	B. REINFORCING STEEL FOR CONCRETE AND MASONRY C. CONCRETE AND/OR MASONRY POST—INSTALLED ANCHORS D. PRECAST CONCRETE COMPONENTS E. CONCRETE FORMWORK FOR STRUCTURAL CONCRETE MEMBERS 120.2 SHOP DRAWINGS TO BE SUBMITTED SHALL PROVIDE COMPLETE INFORMATION FOR THE PRODUCTS OR COMPONENTS TO BE	120.1 THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR REVIEW BY ATLANTIC ENGINEERING SERVICES AND THE PROJECT ARCHITECT. SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL COMPONENTS INCLUDING, BUT NOT LIMITED TO THE FOLLOWING: A. FABRICATED STRUCTURAL STEFF	DETAIL PLUME AND AS RE		STRUCTURES. SUCH DEMOLITION SHAMANNER AS TO MAINTAIN THE STRUCT MAINTAIN THE STRUCT MAINTAIN THE STRUCT AS TO REMAIN. PROVIDE STRUCTURES TO REMAIN. PROVIDE STRUCTURES TO REMAIN. PROVIDE STRUCTURES TO REMAIN PROVIDE STRUCTURES TO REMAIN STRUCTURES.	THE ENGINEERS REVIEW AND APPROVAL TOR SHALL VERIFY ALL EXISTING COND ETC., AND SHALL NOTIFY THE ARCHITEC NICIES, ADDITIONAL INFORMATION, ETC., IE WORK.	STRUCTURAL FEATURES, UNLESS NOTED OTHERWISE. THE ARCHITECTURAL DRAWINGS SHALL GOVERN THE WORK FOR ALL DIMENSIONS. 110.3 THE METHOD AND FREQUENCY OF ATTACHING MECHANICAL EQUIPMENT UNITS, ETC., TO THE STRUCTURAL ELEMENTS SHALL BE	THIS DRAWING HAS BEEN PRENGINEERING SERVICES CADD OR SYMBOLS, OTHER THAN FOR SYMBOLS, MADE WITHOUT THAVE BEEN MADE SERVICES AND ENGINEERING SERVICES AND	10. RESPONSE MOD 11. ANALYSIS PROCI	IGN CATEGORY IIC FORCE	0 20	A. WIND LOADS (IN ACCORDANCE WITH DESIGN BUILDING CODE PER GENERAL NOTE 100.1): 1. BASIC WIND SPEED (3 SECOND GUST) = 90 MPH 2. WIND IMPORTANCE FACTOR (Iw) = 1.0 3. OCCUPANCY CATEGORY = II 4. EXPOSURE CATEGORY = C	3. SNOW EXPOSURE FACTOR, Ce 1.0 4. SNOW LOAD INPORTANCE FACTOR, Is 1.0 5. THERMAL FACTOR, Ct 1.0	TRAINING OCCUPANCY 1 NOW LOADS: BASE GROUND SNOW LOAD, Pg 3	סיים ו	100.1 DESIGN BUILDING CODE: A. INTERNATIONAL BUILDING CODE, 2006 100.2 GRAVITY LOADS:	GENERAL NOTES 100. DESIGN CRITERIA
CONCRETE MIX DESIGN PREPARED IN ACCORDANCE WITH THE SPECIFICATIONS TO THE STRUCTURAL ENGINEER FOR REVIEW. D SYSTEM. ANY OTHER LETTERING, LINES OR SYMBOLS, OTHER THAN PROFESSIONAL STATEMENT OF THE STATEMENT OF	300.18 PRIOR TO PLACING CONCRETE, THE CONTRACTOR SHALL SUBMIT FOR REVIEW BY STRUCTURAL ENGINEER A CONCRETE POUR SCHEDULE SHOWING LOCATION OF ALL PROPOSED CONSTRUCTION JOINTS. 300.19 PRIOR TO CONCRETE PLACEMENT, THE CONTRACTOR SHALL SUBMIT A	OUTSIDE DIAMETER LARGER THAN 1/3 THE SLAB THICKNESS AND SHALL NOT BE SPACED CLOSER THAN 3 DIAMETERS ON CENTERS. ALUMINUM CONDUITS SHALL NOT BE PLACED IN CONCRETE, NO CONDUITS SHALL BE PLACED IN SLAB WITHIN 12"-OF COLUMN-FACE OR FACE OF BEARING WALL. NO CONDUITS MAY BE PLACED IN EXTERIOR SLABS.	SLAB EDGES SHALL BE AS SQUARE AS POSSIBLE WITH A LENGTH—TO—WIDTH RATIO NOT TO EXCEED 1.5. 300.16 CONTRACTOR SHALL VERIFY DIMENSIONS AND LOCATIONS OF ALL SLOTS, PIPE SLEEVES, ETC., AS REQUIRED FOR MECHANICAL TRADES BEFORE CONCRETE IS PLACED. 300.17 PIPES OR CONDUITS PLACED IN SLABS SHALL NOT HAVE AN	300.13 ALL TIES SHALL HAVE 135 DEGREE HOOKS. 300.14 PROVIDE 1/2" PREMOULDED EXPANSION MATERIAL WHERE SLAB ON GRADE IS POURED AROUND COLUMNS AND AGAINST WALLS UNLESS OTHERWISE SHOWN ON DRAWINGS. 300.15 CONTRACTION JOINTS FOR SLABS ON GRADE SHALL BE SPACED NO	OSITION ONCRETE ETE SHA ED WIRI	300.10 TOP BARS IN BEAMS SHALL TERMINATE IN A CLASS "B" TENSION SPLICE OR HOOK AT DISCONTINUOUS END. 300.11 PARALLEL REINFORCEMENT PLACED IN TWO OR MORE LAYERS SHALL HAVE A CLEAR DISTANCE BETWEEN LAYERS OF 1". UPPER LAYER BARS SHALL BE PLACED DIRECTLY ABOVE BARS IN THE BOTTOM	118 91 102 131 101 113 TOP REINFORCEMENT SHALL BE MADE AT MID BOTTOM REINFORCEMENT SHALL BE OVER SUITED OTHERWISE.	3000 PSI SIZE TOP BARS ALL OTHERS TOP BARS ALL OTHERS #3 (#10) 28 22 24 19 #4 (#13) 37 29 32 25 #5 (#16) 47 36 40 31 #6 (#19) 56 43 48 37 #7 (#22) 81 63 70 54 #8 (#25) 93 72 80 62 #9 (#29) 105 81 91 70	B. ALL OTHERS MEST CLASS "B" TENSION, CASE "1" MINIMUM, UNLESS OTHERWISE NOTED 300.8 CLASS "B", CASE "1" TENSION SPLICES IN INCHES, SHALL BE AS FOLLOWS:	F. ELEVATED SLABS 2" FOLLOWS:	A. FOOTINGS & GRADE BEAMS	A. DEFORMED BARS ASTM A615, GRADE 60 B. WELDED WIRE FABRIC ASTM A185 300.6 MINIMUM COVER FOR CAST—IN—PLACE CONCRETE REINF., UNLESS	D. COLUMNS, BEAMS, ELEVATED SLABS ON GRADE 0.45 D. COLUMNS, BEAMS, ELEVATED SLABS 0.45 300.4 ALL CONCRETE SHALL BE NORMAL WEIGHT CONCRETE (MINIMUM 144 PCF) WITH ALL CEMENT CONFORMING TO ASTM C150, TYPE I OR II. MAXIMUM AGGREGATE SIZE SHALL BE 1-1/2" FOR FOOTINGS AND 3/4" FOR WALLS AND SLABS, CONFORMING TO ASTM C33.	C. SLABS ON GRADE 4000 PSI D. COLUMNS, BEAMS, ELEVATED SLABS 4000 PSI 300.3 MAXIMUM WATER TO CEMENTITIOUS MATERIALS RATIO: A. FOUNDATIONS 0.45 B. WALLS 0.45		300.1 ALL REINFORCED CONCRETE WORK SHALL BE IN CONFORMANCE WITH THE "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (ACI 318, LATEST EDITION) AND SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI 301, LATEST EDITION) OF THE AMERICAN CONCRETE	THE OWNER/CONTRACTOR SHALL RETAIN THE SERVICES OF A PROFESSIONAL GEOTECHNICAL ENGINEER, SUBJECT TO THE APPROVAL OF THE ARCHITECT, TO INSPECT THE FOUNDATIONS, BEARING LEVELS, ETC., AND VERIFY THAT THE MATERIAL ON WHICH FOUNDATIONS BEAR HAS AT LEAST THE ABOVE NOTED CAPACITY AS PER 200.1.	BEARING CAPACITY OF 2000 PSF, AS PER 200.1. 210.3 ELEVATIONS SHOWN ON THE DRAWINGS AT WHICH FOUNDATIONS ARE TO BEAR ARE APPROXIMATE. MATERIAL ON WHICH FOUNDATIONS ARE TO BEAR SHALL HAVE AT LEAST THE ABOVE NOTED CAPACITY. ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 3'-6" BELOW FINISHED GRADE.	210.1 FOUNDATIONS HAVE BEEN DESIGNED AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH CRITERIA ESTABLISHED IN THE GEOTECHNICAL REPORT PER 200.1. 210.2 SPREAD FOOTINGS HAVE BEEN DESIGNED TO BEAR ON UNDISTURBED SOILS OR PROPERLY COMPACTED FILL HAVING AN ALLOWABLE	200.5 CONCRETE SLABS ON GRADE HAVE BEEN DESIGNED TO BEAR ON COMPACTED SUBGRADE SOILS OR PROPERLY COMPACTED FILL AS PER 200.1. 210. SHALLOW FOUNDATIONS	200.4 THE CONTRACTOR SHALL USE EXTREME CAUTION DURING EXCAVATION. SUCH EXCAVATION SHALL BE PERFORMED IN SUCH A MANNER AS TO MAINTAIN THE STRUCTURAL INTEGRITY OF ALL EXISTING STRUCTURES TO REMAIN. PROVIDE TEMPORARY SHORING AS REQUIRED.	200.3 THE CONTRACTOR SHALL OBSERVE WATER CONDITIONS AT THE SITE AND TAKE THE NECESSARY PRECAUTIONS TO INSURE THAT THE FOUNDATION EXCAVATIONS REMAIN DRY DURING CONSTRUCTION. PROVIDE FOR DEWATERING AS NECESSARY.	WHERE BACKFILL IS REQUIRED ON BOTH SIDES OF THE WALL, BACKFILL BOTH SIDES SIMULTANEOUSLY WITH A GRADE DIFFERENCE NOT TO EXCEED 2'-0" AT ANY TIME. CONTRACTOR SHALL USE EXTREME CAUTION DURING BACKFILLING TO PREVENT DAMAGE TO FOUNDATION WALLS. THE USE OF HEAVY EQUIPMENT FOR BACKFILLING IS NOT RECOMMENDED.	THEIR GEOTECHNICAL REPORT. 200.2 NO BACKFILLING AGAINST FOUNDATION WALLS SHALL BE PERMITTED UNTIL SUPPORTING STRUCTURAL ELEMENTS HAVE BEEN PLACED AND HAVE BECOME CAPABLE OF FURNISHING THE NECESSARY SUPPORT FOR THE WALLS. PROVIDE TEMPORARY SHORING WHERE REQUIRED.	200. FOUNDATIONS — GENERAL 200.1 FOUNDATIONS HAVE BEEN DESIGNED AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH CRITERIA ESTABLISHED BY L.R. KIMBALL IN
AMPS AND SIGNATURES, HAVE BEEN MADE WITHOUT THE AUTHORIZATION OF ATLANTIC	HOWEVER, THE ENGINEER SHALL BE THE SOLE JUDGE OF ACCEPTANCE AND THE CONTRACTOR'S BID SHALL ANTICIPATE THE USE OF THOSE SPECIFIED DETAILS SHOWN ON THE DRAWINGS THE CONTRACTORTISTRESPONSIBLE FOR THE DESIGN OF SUCH ALTERNATE DETAILS WHICH HE PROPOSES.	SHALL BE SHOWN ON THE SHOP DRAWING AND MADE IN THE SHOP. CUTS OR BURNING OR HOLES IN STRUCTURAL STEEL IN THE FIELD WILL NOT BE PERMITTED. 510.10—ALTERNATE—CONNECTION—DETAILS—MAY—BE—USED—IF—SUCH—DETAILS—ARE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL.	UNLESS NOTED OTHERWISE, FOR AN END REACTION "R" EQUAL TO NOT LESS THAN ONE HALF THE UNIFORM LOAD CAPACITY OF THE MEMBER IN ACCORDANCE WITH AISC SPECIFICATIONS, BUT NOT LESS THAN 6 KIPS. THE EFFECTS OF CONCENTRATED LOADS OCCURRING CLOSE TO THE ENDS OF THE BEAMS SHALL BE CONSIDERED IN THE CONNECTION DESIGN. 510.9 CUTS, HOLES AND COPING, ETC. REQUIRED FOR OTHER TRADES	- z 쿠쿠요ㅋ쿠론	MERG NERG	510.5 ALL BOLTS SHALL BE ASTM A325, 3/4" DIAMETER, UNLESS OTHERWISE NOTED. WHERE NECESSARY DUE TO CONNECTION REQUIREMENTS PROVIDE BOLTS IN CONFORMANCE WITH ASTM A490. DO NOT MIX GRADES OF BOLTS OF THE SAME DIAMETER ON THE PROJECT. 510.6 ALL WELDING SHALL BE IN ACCORDANCE WITH THE STRUCTURAL	510.4 GALVANIZED STRUCTURAL STEEL A. STRUCTURAL SHAPES AND RODS ASTM A123 B. BOLTS, FASTENERS AND HARDWARE ASTM A153	A. STRUCTURAL 'W' SHAPES ASTM A992 B. STRUCTURAL 'M', 'S', 'HP' SHAPES GRADE 50 C. CHANNELS ASTM A572, GRADE 50 D. STEEL TUBES (HSS SHAPES) ASTM A500, GRADE B E. STEEL PIPE (ROUND HSS) GRADE B ASTM A500, GRADE B ASTM A500, GRADE B ASTM A500, GRADE B ASTM A500,	510.2 ALL STRUCTURAL STEEL WORK SHALL BE IN ACCORDANCE WITH ANSI/AISC 360-05 - "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS." 510.3 GRADE OF STEEL	510. STRUCTURAL STEEL 510.1 ALL STRUCTURAL STEEL WORK SHALL BE IN ACCORDANCE WITH THE "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS - ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN" (1989 EDITION, AS REVISED)	REINFORCING. 420.15 MASONRY COURSING SHOWN IN SECTION IS APPROXIMATE, REFER TO PLANS AND ELEVATIONS FOR ACTUAL COURSING, COORDINATE ACTUAL COURSING REQUIREMENTS WITH ARCHITECTURAL DRAWINGS.	420.13 PROVIDE CLEAN OUT AND INSPECTION HOLES AT BOTTOM OF MASONRY WALL IN ACCORDANCE WITH MASONRY CODE AT REINFORCING IF HIGH LIFT GROUTING (OVER 4 FEET HIGH) IS USED. 420.14 DEFORMED BAR REINFORCEMENT SHALL CONFORM TO ASTM A615, GRADE 60. PROVIDE LAP SPLICES OF 48 BAR DIAMETERS MINIMUM. PROVIDE BAR SPACERS AS REQUIRED TO PROPERLY LOCATE	OTHERWISE WHERE CONTROL JOINTS ARE NOT INDICATED ON THE ARCHITECTURAL DRAWINGS, CONTROL JOINTS SHALL BE LOCATED AT A MAXIMUM SPACING OF 24 FEET ON CENTER AND A MAXIMUM OF 10 FEET FROM CORNERS; AT MAJOR HEIGHT CHANGES; CHANGES IN WALL THICKNESS; AND AT WALL OPENINGS. THE CONTRACTOR SHALL SUBMIT THE PROPOSED CONTROL JOINT LAYOUT TO THE ARCHITECT FOR REVIEW AND APPROVAL.	SPACE IS LESS HAW 2. PROVIDE COARSE GROUT WHEN SPACE WIDTHS 2" OR GREATER. PROVIDE FINE GROUT WHEN REINFORCING HAS LESS THAN 1/2" CLEARANCE. 420.12 PROVIDE CONTROL JOINTS IN MASONRY PER THE TYPICAL DETAILS. REFER TO ARCHITECTURAL DRAWINGS FOR CONTROL JOINT LOCATIONS. PROVIDE CONTROL JOINTS IN CONCRETE MASONRY RACKIIP TO MATCH LOCATIONS IN MASONRY VENEER LINI FSS NOTED	420.10 REINFORCED MASONRY UNITS SHALL BE FILLED SOLID WITH 3000 PSI GROUT AT ALL CELLS CONTAINING REINFORCING AND WHERE SHOWN ON PLANS. GROUT ALL CELLS BELOW GRADE SOLID. 420.11 PROVIDE FINE GROUT PER ASTM C476 WHEN WIDTH OF GROUT	COURSES ABOVE AND BELOW A MASONRY OPENING. PROVIDE LAP AS RECOMMENDED BY MANUFACTURER WITH A MINIMUM OF 6". DISCONTINUE JOINT REINFORCING AT CONTROL JOINTS. PROVIDE "L" SHAPE AND "T" SHAPE DUR-O-WAL AT ALL INTERSECTION CORNERS WITH 8" MINIMUM LAP. SEE TYPICAL DETAILS. 420.9 FULL BED AND HEAD JOINTS SHALL BE USED.	420.8 HORIZONTAL JOINT REINFORCING FOR ALL EXTERIOR AND LOAD BEARING WALLS SHALL BE GALVANIZED TRUSS OR LADDER TYPE DUR-0-WAL OR EQUIVALENT AS APPROVED BY THE ENGINEER WITH 2-9 GAGE LONGITUDINAL WIRE AND 9 GAGE CROSS WIRE, SPACED AT 16" CENTER TO CENTER, UNILESS NOTED OTHERWISE. PROVIDE ADDITIONAL LAYERS OF JOINT REINFORCEMENT IN THE FIRST TWO	420.6 PROVIDE SOLID AND HOLLOW LOAD BEARING CONCRETE BLOCK UNITS PER ASTM C90, TYPE N-II, AS REQUIRED TO PROVIDE F'm AS NOTED BELOW. 420.7 MINIMUM 28-DAY ULTIMATE COMPRESSIVE STRENGTH OF MASONRY: A. F'm1500 PSI	D: PORTLANDINFORMING	A. COMPRESSIVE STRENGTH (F'c) OF GROUT = 3,000 PSI MIN. B. SLUMP OF GROUT SHALL BE 8 TO 11 INCHES AS MEASURED ACCORDING TO ASTM C143. C. MAX. AGGREGATE SIZE SHALL BE 3/8" (AGGREGATE GRADED TO PRODUCE FINE GROUT IN CONFORMANCE WITH ASTM C476 AND	MORTAR AT ALL HIGH PSI OR GREATER. MASONRY AND WISE.	TO THE BROBUBTION SPECIFICATION	CONSTRUCTION OF THE LATEST EDITION OF "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI 530) AND THE "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.1) OF THE AMERICAN CONCRETE INSTITUTE WITH THE FOLLOWING ADDITION TO THE REQUIREMENTS OF ACI 530.1 SECTION 1.8—B: FOR ALL CONDITIONS WHEN TEMPERATURES FALL BELOW 40 DEGREES F, THE TEMPERATURE OF THE NEWLY LAID MASONRY OR NEWLY GROUTED	STRUCTURES" (ACI 530) AND THE "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.1) OF THE AMERICAN CONCRETE INSTITUTE. 420.2 ALL MASONRY WORK TO BE EXECUTED IN COLD WEATHER SHALL BE IN CONFORMANCE WITH THE RECOMMENDATIONS FOR COLD WEATHER	420. MASONRY 420.1 ALL MASONRY WORK SHALL BE IN CONFORMANCE WITH THE LATEST FORTION OF "BILLING CODE REQUIREMENTS FOR MASONRY
ENGINEERING SERVICES AND ARE INVALID.															EL BAR GRATING SHALL	535.6 FABRICATE THE STEEL GRATING TO BE REMOVABLE AND INSTALL WITH CLIPS AND FASTENERS AS RECOMMENDED BY THE MANUFACTURER FOR THE INSTALLATION CONDITIONS SHOWN. 535.7 STEEL GRATING MANUFACTURER SHALL SUBMIT SHOP DRAWINGS INDICATING AS A MINIMUM THE PROPOSED JOINT LOCATIONS, CONNECTION DETAILS GRADE OF STEEL AND FINISH	535.4 GRADE OF STEEL FOR STEEL FLOOR PLATE: ASTM A36 TYPICAL, OR ASTM A572 GRADE 50 AS NOTED. 535.5 ALL STEEL GRATING SHALL BE SECURELY FASTENED TO THE SUPPORTING STRUCTURE BY WELDING, BOLTING, OR CLIPS AND FASTENERS AT A SPACING NOT EXCEEDING 24" O.C.	OUTY METAL BAR GW SERIES WITH ON CENTER WITH APABLE OF SUPP 535-1.	BAR GRATING SHALL COMPLY WITH NAAMM MBG GRATING MANUAL FOR STEEL, STAINLESS STEEL, STAINLESS AND STAIR TREADS" FOR NON-HEAVY-I	535. STEEL PLATE FLOOR/STEEL GRATE FLOOR 535.1 ALL STEEL BAR GRATING SHALL BE CAPABLE OF SUPPORTING A 100 PSF SUPERIMPOSED LOAD AT THE INDICATED SPANS. THE DEFLECTION OF THE STEEL BAR GRATING UNDER THE ABOVE LOAD OR A 300 POUND CONCENTRATED LOAD AT MIDSPAN SHALL NOT	ERD-C621 OR ASIM C109, WITH TO OF NOT LESS THAN L ANCHOR BOLTS SHALL BE AND A MINIMUM DIAMETER ED OTHERWISE.	COMPLYING WITH ASTM E164. VELING GROUT SHALL BE NON-SHRINK, NON-METALL CTORY PRE-MIXED GROUT TESTED IN ACCORDANCE WE COMPLY TO THE TEST OF THE TEST O	CONNECTIONS SHALL BE RANDOMLY INSPECTED BY MAGNETIC PARTICLE METHOD, COMPLYING WITH ASTM E109, PERFORMED ON ROOT PASS AND ON FINISHED WELD. D. ONE HUNDRED PERCENT OF FULL PENETRATION WELDS SHALL HAVE ULTRASONIC INSPECTION, COMPLYING WITH ASTM E164. E. ONE HUNDRED PERCENT OF WELDS IN BEAM AND COLUMN E. ONE HUNDRED PERCENT OF WELLS IN BEAM AND COLUMN E. ONE HUNDRED PERCENT OF WELLS IN BEAM AND COLUMN E. ONE HUNDRED PERCENT OF WELLS IN BEAM AND COLUMN E. ONE HUNDRED PERCENT OF WELLS IN BEAM AND COLUMN E. ONE HUNDRED PERCENT OF W	TS SHALL BE TESTED PERISUALLY INSPECTED.	510.11 ALL STRUCTURAL STEEL FRAMES SHALL BE SECURELY BRACED UNTIL ALL FLOOR SLABS, ROOF DECKS AND SHEAR WALLS HAVE BEEN INSTALLED AND BECOME CAPABLE OF STABILIZING THE FRAMES.
AES P																				SEE "CONTINUOUS WALL FOOTING SCHEDULE"	INDICATES NEW BEAM OR	INDICATES STEP IN FOOTING SEE "TYPICAL DETAILS" INDICATES DIRECTION OF SPAN	INDICATES COMPACTED FILL OR BEARING STRATA PER GEO-TECHNICAL REPORT INDICATES GRANULAR FILL	INDICATES NEW CONCRETE INDICATES GROUT IN SECTIONS	LEGEND
ROJECT #07-114 AES PLOT DATE: 04.30.07		GENERAL	CHECKED BY: RSB © L. Robert Kimball & Associates Architects and Engineers, Inc.	MARK DATE DESCRIPTION ISSUE: 05 / 02 / 07 PROJECT NO: 07-2200-0175 CAD DWG FILE: B IS			B KEYPLAN			YOUNGWOOD	OWNER: WESTMORI	c COLLEG	MEST	PROJECT NAME		WORKER STONE	D PROFESSIONAL		AES			web site CONSULTANTS	615 West Highland Av Ebensburg, PA 1593 phone (814) 472–7700 fax (814) 472–7712 email gebldgs@irkimball.co	Robert Chitects	



L. Robert Kimball & Associates Architects and Engineers, Inc.

